

Section 2



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Basic Understanding of Traffic Control at Intersections

A. Signs

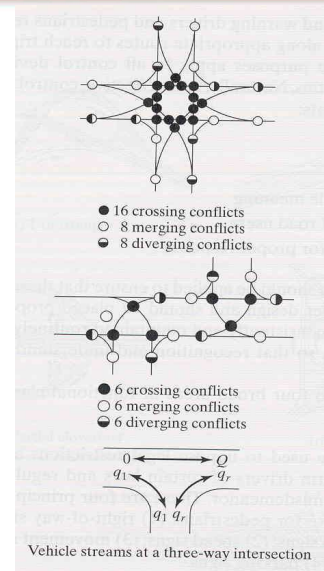
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Intersections...

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- Reduce **conflicts** between road users
- Improve efficiency and safety
- Consider
 - Human factors
 - Traffic
 - Geometrics
 - Economics



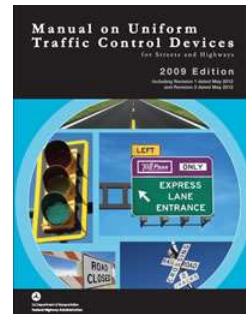
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Requirements

- To be effective, a traffic control device should meet five basic requirements:
 - Fulfill a need;
 - Command attention;
 - Convey a clear, simple meaning;
 - Command respect from road users;
 - Give adequate time for proper response.



- *National Standard*
- *Legal Document*



3

3

Signs




- Traffic signs convey information through their shape, color, message, and placement.
- Standard sign shapes:
 - Octagon -- Exclusively for STOP signs
 - Equilateral Triangle, Point Down -- Exclusively for YIELD signs
 - Circle -- Exclusively for Railroad Advance Warning and Civil Defense Evacuation Route marker
 - Pennant -- Exclusively for NO PASSING ZONE signs


4


4


Signs



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- Standard Sign Shapes
 - Diamond -- Used for warning signs
 - Rectangle, Longer Dimension Vertical -- Used for regulatory signs
 - Rectangle, Longer Dimension Horizontal -- Used for guide signs


5


5


Signs




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- Other Traffic Signs
 - Trapezoid -- Used for recreational area guide signs
 - Pentagon -- Used for school advance warning signs
 - Crossbuck -- Used for railroad crossing signs
 - Other shapes -- Used for route marker signs









6

6

Sign Colors



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	<ul style="list-style-type: none"> • Red 		<ul style="list-style-type: none"> • Yellow
	<ul style="list-style-type: none"> • Black 		<ul style="list-style-type: none"> • Brown
	<ul style="list-style-type: none"> • White 		<ul style="list-style-type: none"> • Green
	<ul style="list-style-type: none"> • Orange 		<ul style="list-style-type: none"> • Blue

7

7

Sign Colors



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- **Fluorescent Yellow-Green** -- Now approved for pedestrian, bicycle, and school crossing warning signs
- **Fluorescent Pink** – Incident Management Signs
- **Purple** -- Reserved for future use
- **Light Blue** -- Reserved for future use
- **Coral** -- Reserved for future use

Figure 6I-1. Examples of Traffic Incident Management Area Signs



W3-4



W4-2



W9-3



E5-2a



M4-8a



M4-9



M4-10

8

8

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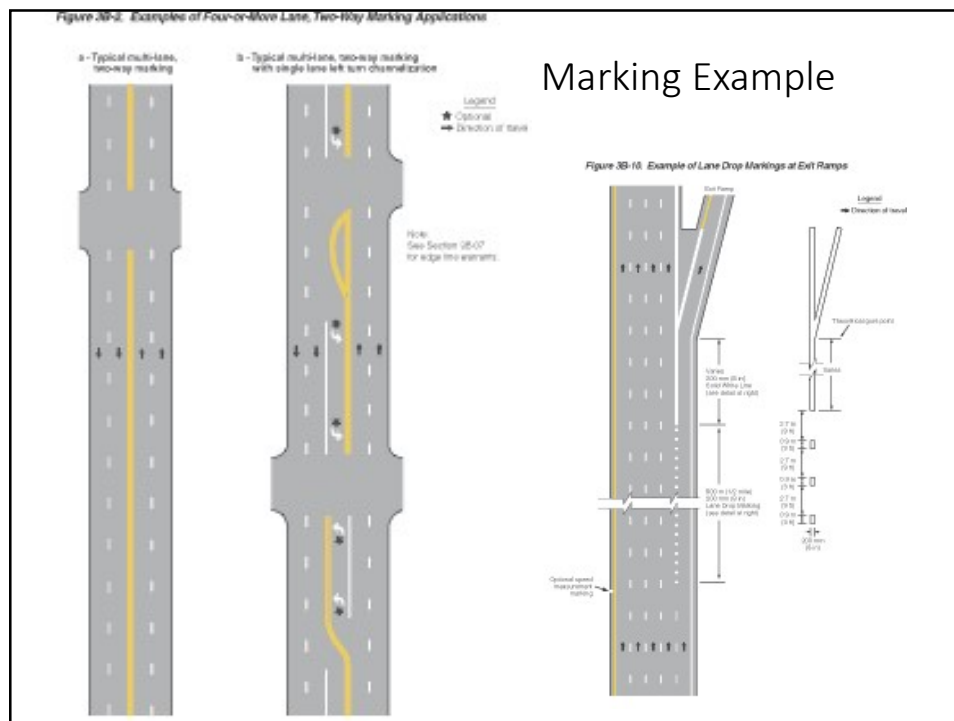
Basic Principles of Traffic Control

B. Markings

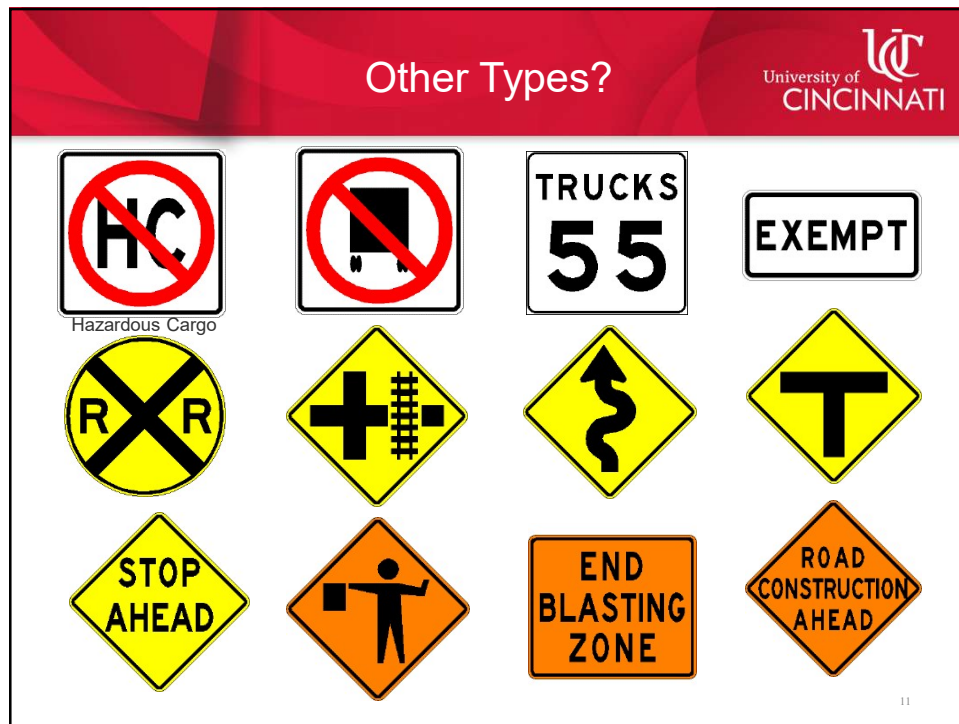
- See PART III of the MUTCD
 - Pavement lines and symbols
 - Object Markers
 - Delineators

9

9



10



11

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Basic Understanding of Traffic Control at Intersections

C. Signalization

12

12

Traffic Signals



- **The MUTCD defines nine types of traffic signals**

- Traffic control signals
- Pedestrian signals
- Emergency vehicle traffic signals
- Traffic control signals for one-lane, two-way facilities
- Traffic control signals for freeway entrance facilities (ramp metering)
- Traffic control signals for movable bridges
- Lane-use control signals
- Flashing beacons
- In-roadway light

13

13

Traffic Signals



- **How do we decide if needed ?**

- ✓ Traffic Engineering Studies
- ✓ Warrants

- **Traffic signals advantages**

- ✓ Provide for the orderly movement of traffic
- ✓ Increase capacity
- ✓ Improve safety (reduce certain crash types)
- ✓ Coordinate traffic flow for continuous movement
- ✓ Introduce gaps


14

14

Traffic Signals

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- **However, traffic signals are not the solution to all urban intersection problems!!**
- **Traffic signals disadvantages**
 - Add delay
 - Encourage disobedience
 - Send volume to other routes
 - Increase certain types of crashes
 - Safety



15

15

Terminology and Definitions

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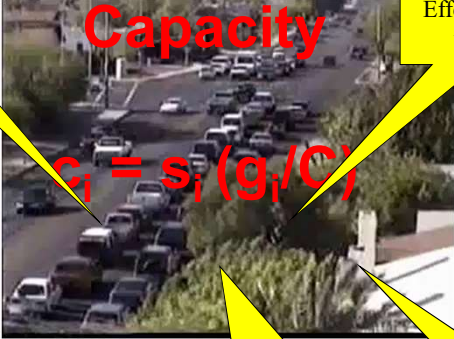
- Cycle: One complete sequence of signal indications (C).
- Cycle length: Time (sec) to complete one full cycle.
- Phase: Part of a cycle allocated to any combination of traffic movements receiving the right of way (R/W) simultaneously during one or more intervals.
- Interval: a period of time during which none of the signal indications change.
- Green Time/Interval: Green indication time.
- Change Interval: The yellow indication for a given movement
- Clearance Interval: The "all-red" indication for a given movement.
- Lost Time: Time during which the intersection is not effectively used by any movement; Start-up (I_1), Clearance (I_2).

16

16

Capacity of a Signalized Approach

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Capacity

$C_i = s_i (g_i/C)$

Capacity of lane group i (veh/h)

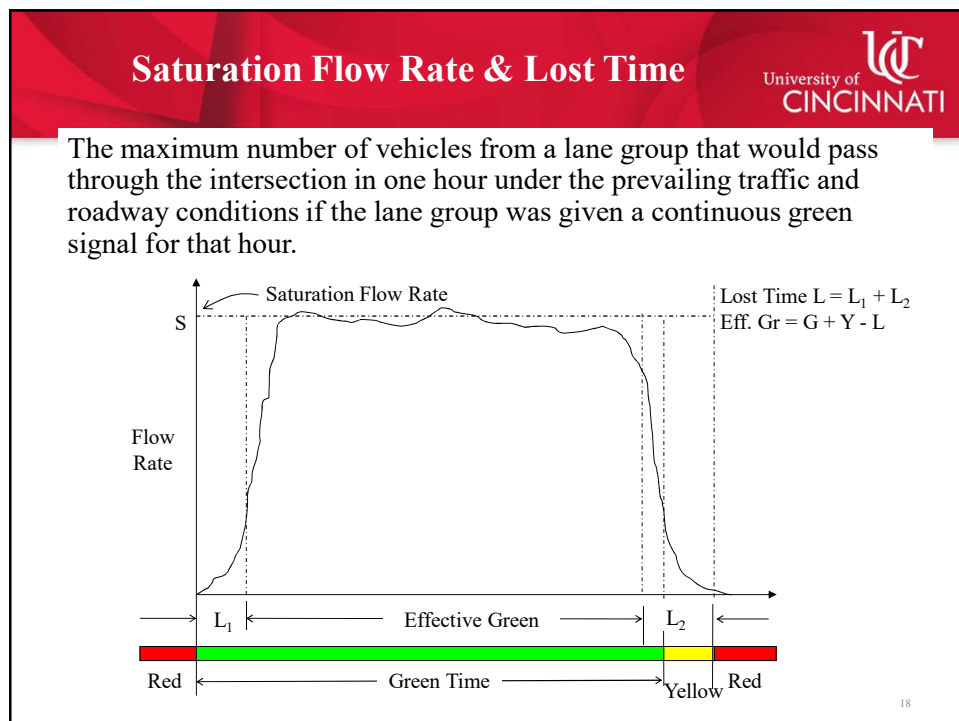
Effective green time, sec

Saturation flow rate for lane group i (veh/h)

Signal cycle time, sec

g_i/C = “green ratio” = actual time available

17



18

Saturation Flow Rate Defined in Highway Capacity Manual



Ideal saturation flow:
2000veh/h/ln for metropolitan areas; otherwise 1900 veh/h/ln

$$S = S_0 N f_w f_{HV} f_g f_p f_a f_{bb} f_{Lu} f_{RT} f_{LT} f_{Lpb} f_{Rpb}$$

Diagram illustrating the factors in the saturation flow rate equation $S = S_0 N f_w f_{HV} f_g f_p f_a f_{bb} f_{Lu} f_{RT} f_{LT} f_{Lpb} f_{Rpb}$:

- S_0 : Ideal saturation flow (2000 veh/h/ln for metropolitan areas; otherwise 1900 veh/h/ln)
- N : # of lanes
- f_w : lane width
- f_{HV} : heavy vehicles
- f_g : approach grade
- f_p : parking lane
- f_a : area type
- f_{bb} : bus blocking
- f_{Lu} : lane utilization
- f_{RT} : right turn
- f_{LT} : left turn
- f_{Lpb} : ped for left turn
- f_{Rpb} : ped for right turn

19

19

Capacity of a Signalized Approach



- **Example:** Given a signalized intersection:
Saturation headway for EB approach (two lanes), $h = 2.2$ sec; Cycle length, $C = 80$ sec; Green time for EB approach, $G_i = 37$ sec; Start-up lost time, $l_1 = 2$ sec; Clearance lost time $l_2 = 1$ sec; Yellow time = 3.5 sec.
Capacity of EB approach = ?
- **Solution:**
Saturation flow rate of EB, $s_i = 3600/h = 3600/2.2 = 1636$ veh/hr/ln
Total lost time/phase $t_{Li} = l_1 + l_2 = 2.0 + 1.0 = 3$ sec
Effective green time $g_i = G_i + Y_i - t_{Li} = 37.0 + 3.5 - 3.0 = 37.5$ sec
Capacity of EB approach, c_i (veh/hr/ln)

veh/ln/hr

For the two-lane approach: $c = 767 \times 2 = 1534$ veh/hr

$$c_i = s_i \frac{g_i}{C} = 1636 \frac{37.5}{80} = 767$$

20

20

Saturation Flow Rate

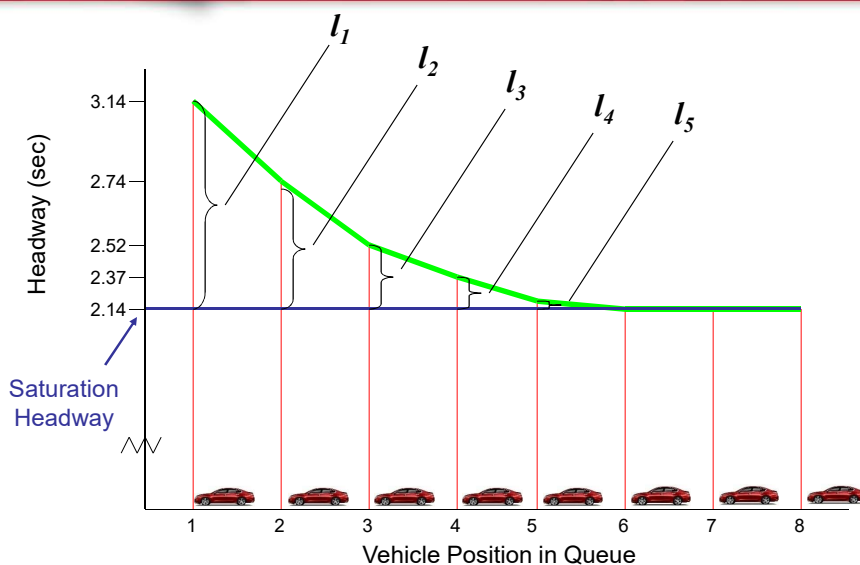
Table 20.2: Saturation Flow Rates from a Nationwide Survey

Item	Single-Lane Approaches	Two-Lane Approaches
Number of Approaches	14	26
Number of 15-Minute Periods	101	156
Saturation Flow Rates		
Average	1,280 veh/hg/ln	1,337 veh/hg/ln
Minimum	636 veh/hg/ln	748 veh/hg/ln
Maximum	1,705 veh/hg/ln	1,969 veh/hg/ln
Saturation Headways		
Average	2.81 s/veh	2.69 s/veh
Minimum	2.11 s/veh	1.83 s/veh
Maximum	5.66 s/veh	4.81 s/veh

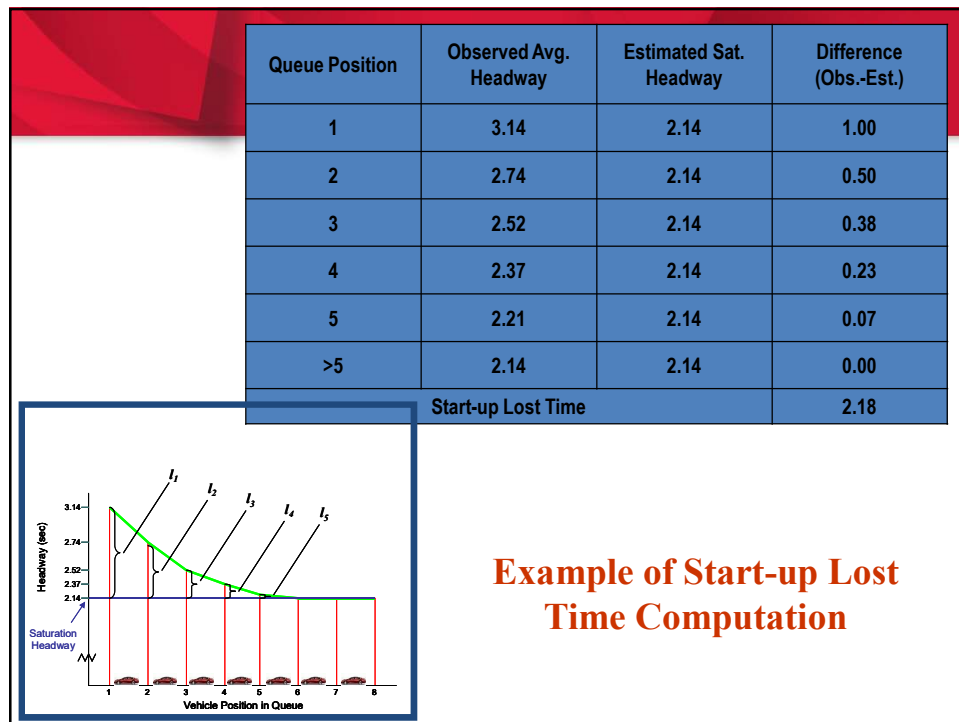
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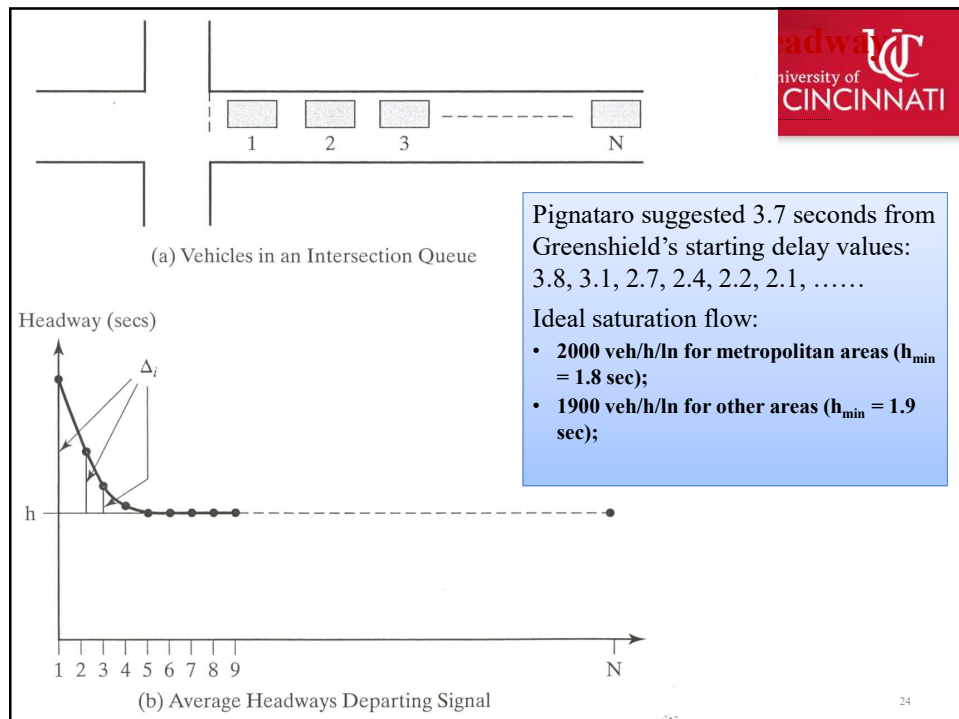
Start-Up Lost Time and Discharge Headway



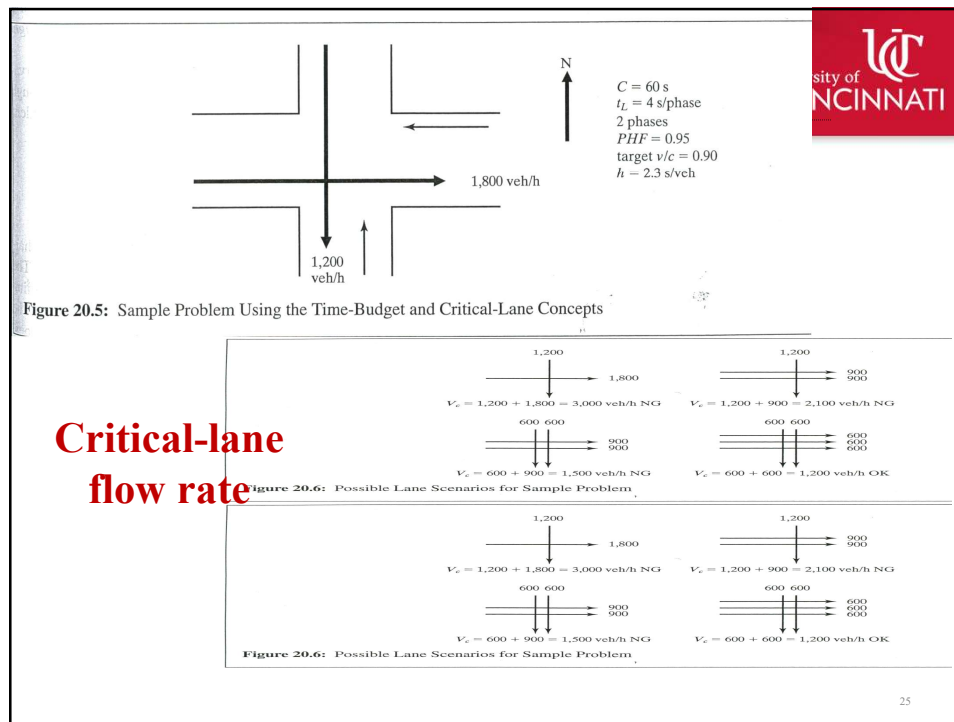
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23



24



**Critical-lane
flow rate**

25

Capacity of a Signalized Intersection



- Critical Volume: the largest movement among all movements within a lane group.
- Steps to determine the capacity:
 - Step 1: calculate total lost time per cycle: $L = N \cdot t_L$
 - N = # of phases in the cycle; t_L = total lost time/phase
 - Step 2: calculate total lost time in one hour, L_H
 - $L_H = N \cdot t_L \cdot (3600/C)$
 - Step 3: calculate green time allocation, T_G
 - $T_G = 3600 - L_H = 3600 - N \cdot t_L \cdot (3600/C)$
 - Step 4: determine capacity V_c given saturation headway, h , and a phase plan
 - $V_c = (3600 - L_H)/h = 1/h[3600 - N \cdot t_L \cdot (3600/C)]$

26

26

Capacity of a Signalized Intersection

- **Example:** a signalized intersection with 3 phases
 Cycle length, $C = 90$ sec
 Lost time per phase $t_L = 3.5$ sec
 Saturation headway, $h = 2$ sec
 Steps to determine the capacity (as shown in last slide)
- **Solution:**

$$V_c = (3600 - L_H)/h = 1/h[3600 - N \cdot t_L \cdot (3600/C)] =$$

$$\frac{1}{2}[3600 - 3 \cdot 3.5 \cdot (3600/90)] = 1590 \text{ veh/hr}$$

27

27

Left-Turn (and Right-Turn) Equivalent

- A left-turn vehicle will consume more effective green time traversing the intersection than a similar through vehicle.
- Example: It was observed that the effective green time allows 11 through vehicles to pass the intersection. One observation suggests that 5 through vehicles plus 2 left-turn vehicles passed the intersection during the effective green time (shown by the following figure), what's the left-turn equivalent factor?

Solution: $11 = 5 + 2E_{LT} \rightarrow E_{LT} = (11-5)/2 = 3.0$. that means, 1 left-turn vehicle's passage time at an intersection allows 3 through vehicles to pass the intersection.

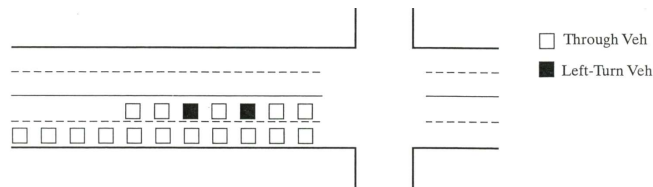


Figure 20.7: Sample Equivalence Observation on a Signalized Intersection Approach

28

28